

GENERAL NOTES

2019 CALIFORNIA BUILDING CODE GOVERN DESIGN AND CONSTRUCTION.
 THESE GENERAL NOTES SHALL APPLY TO ALL SHEETS IN THIS SET OF PLANS.
SAFETY REGULATIONS - CALIF. ADMIN. CODE, TITLE 8, GENERAL SAFETY ORDERS (CAL OSHA) IS APPLICABLE TO THE CONSTRUCTION OF THIS PROJECT AND PROVISIONS THEREOF MUST BE FOLLOWED. ENGR. & COMPANY ENGINEERS IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION, NOR FOR SAFETY ON THE JOBSITE. THESE RESPONSIBILITIES ARE INTENDED TO BE AND TO REMAIN SOLELY THOSE OF THE BUILDER.

ALL DIMENSIONS WHICH ARE DEPENDENT ON EXISTING CONDITIONS SHALL BE FIELD VERIFIED PRIOR TO THE COMMENCEMENT OF CONSTRUCTION.

POWDER ACTUATED PINS IN WOOD SILLS SHALL BE HLTI X-U PB (OR EQUAL) WITH .78" SQUARE METAL WASHER. TWO PINS SHALL BE PLACED 6" AND 10" RESPECTIVELY FROM EACH END OF SILL PLATES. MAXIMUM SPACING BETWEEN PINS SHALL BE 18" O.C.. DO NOT INSTALL PINS UNTIL THE CONCRETE HAS CURED FOR AT LEAST 7 DAYS AND HAS A MINIMUM STRENGTH OF 2000 PSI. PINS ARE .157" (MIN) DIAMETER AND MUST PENETRATE THE CONCRETE AT LEAST 1-1/8".

DRYPACK: 1 PART PORTLAND CEMENT, 2 PARTS PLASTER SAND AND JUST ENOUGH WATER FOR COHESION. DRYPACK MUST BE FIRMLY TAMPED WITH NO VOIDS. DO NOT USE DRYPACK THAT HAS BEEN MIXED LONGER THAN 30 MINUTES.

FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH THE FOLLOWING:

- CONTINUOUS FOUNDATIONS:
 - ALLOWABLE BEARING PRESSURE (D+L): 1,800 PSF
 - ALLOWABLE BEARING PRESSURE (EQ./WIND): 2,400 PSF
 - MINIMUM CONTINUOUS FOOTING WIDTH: 12 INCHES
 - MINIMUM CONTINUOUS FOOTING WIDTH: 18 INCHES
- ISOLATED PAD FOUNDATIONS:
 - ALLOWABLE BEARING PRESSURE (D+L): 2,000 PSF
 - ALLOWABLE BEARING PRESSURE (EQ./WIND): 2,667 PSF
 - MINIMUM CONTINUOUS FOOTING WIDTH: 24 INCHES
 - MINIMUM CONTINUOUS FOOTING WIDTH: 24 INCHES
- ALLOWABLE BEARING PRESSURE INCREASES FOR SIZES BEYOND MINIMUM:
 - INCREASE FOR EACH ADDITIONAL 6" OF DEPTH: 200 PSF
 - INCREASE FOR EACH ADDITIONAL 12" OF WIDTH: 200 PSF
 - MAXIMUM ALLOWABLE BEARING PRESSURE: 2,500 PSF
- LATERAL PRESSURES/FRICTION:
 - ALLOWABLE LATERAL BEARING PRESSURE: 250 PCF
 - (NEGLECT UPPER ONE FOOT OF SOIL UNLESS CONFINED BY SLAB)
 - (PASSIVE RESISTANCE + 2/3 FRICTIONAL RESISTANCE PERMITTED)
 - COEFFICIENT OF STATIC FRICTION: 0.25
 - ACTIVE PRESSURE: 45 PCF
 - EQUIVALENT ACTIVE PRESSURE BACKFILL OF 2:1 RATIO: 60 PCF
 - AT-REST PRESSURE: 60 PCF
- INTERIOR CONCRETE SLAB-ON-GRADE:
 - MINIMUM THICKNESS: 5 INCHES
 - REINFORCEMENT EACH WAY: #3 @ 16" O.C.
 - MINIMUM VAPOR RETARDER: 15 MIL.

THIS IS IN ACCORDANCE WITH THE SOILS REPORT PREPARED BY BRUIN GEOTECHNICAL SERVICES, INC., PROJECT NO.: 20-133, DATED: NOVEMBER 19, 2020.

CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI MINIMUM IN 28 DAYS. THE MAXIMUM WATER-TO-CEMENTitious MATERIAL SHALL BE 0.58 FOR THE CONCRETE MIX DESIGN. ENSURE TO USE CEMENT TYPE II. IF IMPROVED SOIL IS USED PER GEOTECHNICAL REPORT, ENSURE THE SOLUBLE SULFATES ARE LESS THAN 150 PPM OR RECEIVE AN UPDATED CEMENT SPECIFICATION FROM THE RESPONSIBLE ENGINEER OF RECORD TO ENSURE THE CONCRETE IS PROTECTED FROM ANY SULFATE DAMAGE (DESIGN IS BASED ON A COMPRESSIVE STRENGTH OF 2500 PSI IN 28 DAYS. THEREFORE SPECIAL INSPECTION IS NOT REQUIRED.)

CRACK CONTROL IN CONCRETE SLABS - SAW CUTS OR CONSTRUCTION JOINTS SHALL BE MADE IN A PATTERN NOT LARGER THAN 12.5' x 12.5' (PREFERABLY CLOSER). THE SAW CUTS ARE TO BE 1" DEEP FROM THE TOP OF THE CONCRETE SLAB WITH CUTOFF-SAW AND ARE TO BE MADE AS SOON AS PRACTICABLE. AT CONSTRUCTION JOINTS, USE 3/4" DIAMETER BY 2'-6" LONG GREASED DOWELS AT 24" O.C. WITH CENTER OF DOWEL PLACED AT CENTER OF JOINT AND AT MID-DEPTH OF SLAB. OWNER MAY ELECT TO HAVE NO CRACK CONTROL JOINTS.

STRUCTURAL STEEL - ALL PLATES AND SHAPES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A36 OR ASTM A992. PIPE SHALL CONFORM TO THE REQUIREMENTS OF ASTM A53 GRADE B. STRUCTURAL TUBING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A500, GRADE B. BOLTS SHALL BE ASTM A307 UNLESS THE PLANS SHOW OTHERWISE. BOLT HOLES SHALL BE 1/16" LARGER IN DIAMETER THAN THE BOLT.

WELDING SHALL BE DONE BY A CERTIFIED WELDER USING THE SHIELDED ARC PROCESS AND L70 SERIES, LOW HYDROGEN ELECTRODES. WELDS SHALL BE FULL SECTION, FULL PENETRATION AND SHALL DEVELOP THE FULL STRENGTH OF THE SMALLER OF THE PARTS JOINED UNLESS THE PLANS SHOW OTHERWISE.

REINFORCING STEEL #4 AND SMALLER BARS SHALL BE INTERMEDIATE GRADE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 40 (FY=40,000 PSI MINIMUM). #5 AND LARGER BARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60 (FY=60,000 PSI MINIMUM).

LUMBER - BEAMS, HEADERS, AND POSTS ARE TO BE NO. 1 (OR BETTER) D.F.. ALL OTHER WOOD FRAMING IS TO BE NO. 2 (OR BETTER) D.F.. WOOD SILL PLATES ON CONCRETE SHALL BE PRESSURE TREATED D.F.. SILL BOLTS SHALL BE SPACED AS SHOWN ON THE PLANS AND WITHIN 12" OF THE ENDS OF EACH SILL PIECE. NAILING OF LIGHT FRAMING IS TO BE DONE IN ACCORDANCE WITH CBC TABLE 2304.10.1. BOLT HOLES SHALL BE 1/16" LARGER IN DIAMETER THAN THE BOLT. PLACE 3" x 3" x 3/4" WASHER UNDER HEAD AND NUT BEARING ON WOOD. THE WASHER MAY BE SLOTTED PER CBC SECTION 2308.3.1.1.

PLYWOOD SHEAR WALLS SHALL BE 1/2" UNSANDED DOUGLAS FIR PLYWOOD, STRUCTURAL I RATED SHEATHING, SPAN INDEX 32/16, EXPOSURE I, C-D INTERIOR WITH EXTERIOR GLUE, ALL EDGES BLOCKED NAIL WITH 100 COMMON NAILS. SEE SHEAR WALL SCHEDULE FOR NAILING AND SPECIFICATION (ENSURE TO PROVIDE TWO-SIDED SHEATHING WHERE REQUIRED AND STAGGER THE PANEL EDGES AS DETAILED, SEE SHEAR WALL SCHEDULE).

ALL PLYWOOD SHEATHING SHALL BE SUPPORTED ON FRAMING MEMBERS 2" (NOM.) OR WIDER. ADJOINING PANELS SUPPORTED ON THE SAME MEMBER SHALL BUTT AT ITS CENTERLINE. NAILS SHALL BE PLACED NOT LESS THAN 3/8" FROM THE PANEL EDGE. "SHINERS" WILL NOT BE PERMITTED. PLYWOOD ROOF PANELS ARE TO BE LAID WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. JOINTS STAGGERED. NAILS SHALL BE DRIVEN FLUSH BUT SHALL NOT FRACTURE THE SURFACE OF THE PLYWOOD.

ROOF SHEATHING OVER RED-BUILT JOISTS SHALL BE 3/4" MINIMUM UNSANDED DOUGLAS FIR PLYWOOD, A.P.A. RATED EXPOSURE I, C-D INTERIOR WITH EXTERIOR GLUE, INDEX 48/24, TONGUE AND GROOVE EDGES, BLOCKED, NAIL WITH 100 COMMON NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND DIAPHRAGM BOUNDARIES AND @ 12" O.C. AT INTERMEDIATE SUPPORTS (ENSURE NAILING IS IN CONTACT WITH FRAMING MEMBERS - 1/4" IN WIDTH OR WIDER).

ROOF SHEATHING OVER DIMENSIONAL LUMBER (2x ROOF JOISTS) SHALL BE 15/32" UNSANDED DOUGLAS FIR PLYWOOD, A.P.A. RATED EXPOSURE I, C-D INTERIOR WITH EXTERIOR GLUE, INDEX 32/16 OR 24/0, UNBLOCKED, NAIL WITH 80 COM. @ 6" O.C. AT SUPPORTED PANEL EDGES AND AT DIAPHRAGM BOUNDARIES AND @ 12" O.C. AT INTERMEDIATE SUPPORTS.

ROOFING - CLASS A OR B COMPO BUILT-UP ROOFING (USING NO ROCK).

5/8" GYPSUM WALLBOARD SHALL BE NAILED WITH 6D WALLBOARD NAILS AT 7" O.C. AT SUPPORTED PANEL EDGES AND ALL INTERMEDIATE BEARINGS.

ACoustic GRID CEILING WHEN SHOWN ON THE PLANS SHALL BE 2" x 4" T-BAR GRID SYSTEM WITH TRIM AT ALL VERTICAL SURFACES. BAR SYSTEMS: EASTERN OR EQUAL WHITE FINISH, WITH #12 WIRE HANGERS. LAY-IN BOARD SHALL BE 2" x 4" FISSURED 9/16" MINERAL BOARD, CLASS A, FACTORY WHITE FINISH.

PREFABRICATED WOOD JOISTS - DESIGN, ENGINEERING AND FABRICATION SHALL BE THE RESPONSIBILITY OF THE JOIST SUPPLIER. THEY SHALL BE SPACED AS SHOWN ON THE PLANS AND SHALL BE DESIGNED TO CARRY ALL IMPOSED LOADS (DEAD AND LIVE, AS SHOWN ON THE PLANS AND AS REQUIRED BY APPLICABLE CODES). SPANS, OVERHANGS, SUPERSTRUCTURES, ETC. SHALL BE AS SHOWN ON THE PLANS. JOISTS SHALL BE ADDED AS NECESSARY TO SUPPORT ROOF-MOUNTED EQUIPMENT. THE JOIST DRAWINGS, COMPUTATIONS AND SPECIFICATIONS SHALL BE STAMPED AND SIGNED BY A CIVIL ENGINEER COMPETENT IN HIS FIELD AND CURRENTLY LICENSED IN CALIFORNIA. THE JOIST MANUFACTURER SHALL SUPPLY NECESSARY RECOMMENDATIONS FOR BRACING JOISTS DURING AND AFTER ERECTION. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO INSTALL THE JOISTS PLUMB AND PROPERLY ALIGNED AT THE PROPER SPACING AND WITH PROPER SUPPORTS. HE SHALL ALSO PROVIDE AND INSTALL BRACING AS RECOMMENDED BY THE MANUFACTURER. WEB STIFFENERS WHERE REQUIRED AS DETAILED AND FURNISHED BY THE JOIST MANUFACTURER. THE JOISTS ARE TO BE TRANSPORTED AND HANDLED IN SUCH A WAY AS TO AVOID DAMAGE. IN THE EVENT OF JOIST DAMAGE RESULTING FROM HANDLING, TRANSPORTING OR ANY OTHER CAUSE, THE CONTRACTOR MUST IMMEDIATELY NOTIFY THE JOIST MANUFACTURER SO THAT CORRECTIVE PROCEDURES MAY BE PRESCRIBED.

PARALLAM BEAMS (PLB) ARE TO BE 2.2E PARALLAM PSL MANUFACTURED BY TRUS JOIST WEYERHAEUSER OR APPROVED EQUAL.

STATEMENT OF SPECIAL INSPECTIONS

Testing Agency:	Date:	
Project Name:	Owner:	Rancho Vista Shopping Center
Project Description:		Wood Framed Shopping Center
Project Location:	City:	Palmdale, CA 93551
R.D.P. of Record:	Engel & Co. Job No.:	23423
Architect:	N/A	
Soils Report by:	Bruin Geotechnical Services, Inc. File No.:	20-133 Date: November 19, 2020

The special inspector shall refer to the approved construction drawings, CBC 2019 Chapter 17 and the documents referenced therein to verify the following selected items:

Frequency of inspection; check all applicable conditions	REMARKS
Continuous Periodic	
CONCRETE CONSTRUCTION (SEE CBC 2019 TABLE 1705.3)	
<input type="checkbox"/> 1. Inspect reinforcement, including prestressing tendons, and verify placement.	
<input type="checkbox"/> 2. Reinforcing bar welding: <ul style="list-style-type: none"> a. Verify weldability of reinforcing bars other than ASTM A706; b. Inspect single-pass fillet welds, maximum 5/16"; 	
<input type="checkbox"/> 3. Inspect anchors cast in concrete.	
<input type="checkbox"/> 4. Inspect anchors post-installed in hardened concrete members. <ul style="list-style-type: none"> a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads b. Mechanical anchors and adhesive anchors not defined in 4.a. 	
<input type="checkbox"/> 5. Verify use of required design mix.	
<input type="checkbox"/> 6. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	
<input type="checkbox"/> 7. Inspect concrete and shotcrete placement for proper application techniques.	
<input type="checkbox"/> 8. Verify maintenance of specified curing temperature and techniques.	
<input type="checkbox"/> 9. Inspect prestressed concrete for: <ul style="list-style-type: none"> a. Application of prestressing forces; b. Growing of bonded prestressing tendons; 	
<input type="checkbox"/> 10. Inspect erection of precast concrete members.	
<input type="checkbox"/> 11. Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	
<input type="checkbox"/> 12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	

SOILS INSPECTIONS (SEE CBC 2019 TABLE 1705.6)

- 1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.
- 2. Verify excavations are extended to proper depth and have reached proper material.
- 3. Perform classification and testing of compacted fill materials.
- 4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.
- 5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.

STRUCTURAL WOOD (SEE CBC 2019 TABLE 1705.12)

- 1. Verify sheathing, panel edge member thickness and location, nail spacing and sill bolts at all shear walls with edge nails spaced at 4 inches on center or less.
- 2. Verify sheathing, panel edge support, number of fastener lines and nail spacing at all floor and/or roof diaphragms with edge nails spaced at 4" o.c. or less.
- 3. Verify the location, size and placement of anchor bolts used in conjunction with holdowns at shear walls. Also verify attachment of the holdown to the wood boundary member.
- 4. Verify the location, size and placement of chords and collectors used in conjunction with shear walls and diaphragms. Verify chord and collector splices and attachment of the chords and collectors to other structural elements of the seismic force resisting system.

Ensure the proper fastening of each sheathing on two-sided shear walls.

MISCELLANEOUS ITEMS

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SPECIAL INSPECTION NOTES:

Copies of all necessary test and inspection records shall be filed with building official overseeing the project, as well as the registered design professional in responsible charge of the project, the engineer in charge of structural design and the soils engineer.

All test specimens shall be consistent with the materials, workmanship and details to be used throughout this project.

The contractor responsible for overseeing the construction of the main lateral force resisting systems on this project shall submit a written statement of responsibility to the building official and the owner prior to the commencement of work on that particular system that shall contain the following (per CBC 2019 1704.4):

1. Acknowledgment of awareness of the special requirements contained in the statement of special inspections
2. Acknowledgment that control will be exercised to obtain conformance with the construction documents approved by the building official
3. Procedures for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of reports
4. Identification and qualifications of the person(s) exercising such control and their position(s) in the organization.

It shall be the responsibility of the contractor to read and understand the above inspection requirements and to coordinate the testing schedule with the appropriate testing agency.

STATEMENT OF SPECIAL INSPECTIONS (cont'd)

INSPECTION OF HIGH STRENGTH BOLTING:

TABLE N6.6-1	QC	QA
Inspection Tasks Prior to High Strength Bolting		
Manufacturer's certifications available for fastener materials	QC	QA
Fasteners marked in accordance with ASTM requirements	O	O
Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	O	O
Proper bolting procedure selected for joint detail	O	O
Connecting elements, including the appropriate firing surface condition and hole preparation, if specified, meet applicable requirements	O	O
Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	P	O
Proper storage provided for bolts, nuts, washers and other fastener components	O	O

TABLE N6.6-2	QC	QA
Inspection Tasks During High Strength Bolting		
Fastener assemblies, of suitable condition, placed in all holes and washers (if required) are positioned as required	O	O
Joint brought to the snug-tight condition prior to the pretensioning operation	O	O
Fastener component not turned by the wrench prevented from rotating	O	O
Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free ends	O	O

TABLE N6.6-3	QC	QA
Inspection Tasks After High Strength Bolting		
Document acceptance or rejection of bolted connections	P	P

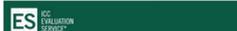
INSPECTION OF COMPOSITE CONSTRUCTION:

TABLE N6.1	QC	QA
Inspection of Steel Elements of Composite Construction Prior to Concrete Placement		
Placement and installation of steel deck	P	P
Placement and installation of steel headed stud anchors	P	P
Document acceptance or rejection of steel elements	P	P

Quality control (QC) inspection tasks shall be performed by the fabricator's or erector's quality control inspector (QCI), as applicable, in accordance with Sections N5.4, N5.6 and N5.7. Tasks in Tables N5.4-1 through N5.4-3 and Tables N6.6-1 through N6.6-3 listed for QC are those inspections performed by the QCI to ensure that the work is performed in accordance with the construction documents. For QC inspection, the applicable construction documents are the shop drawings and the erection drawings, and the applicable referenced specifications, codes and standards.

Quality assurance (QA) inspection of fabricated items shall be made at the fabricator's plant. The quality assurance inspector (QAI) shall schedule this work to minimize interruption to the work of the fabricator. QA inspection of the erected steel system shall be made at the project site. The QAI shall schedule this work to minimize interruption to the work of the erector. The QAI shall review the material test reports and certifications as listed in Section N3.2 for compliance with the construction documents. QA inspection tasks shall be performed by the QAI, in accordance with Sections N5.4, N5.6 and N5.7. Tasks in Tables N5.4-1 through N5.4-3 and N5.6-1 through N5.6-3 listed for QA are those inspections performed by the QAI to ensure that the work is performed in accordance with the construction documents.

O - Observe these items on a random basis. Operations need not be delayed pending these inspections.
P - Perform these tasks for each welded joint or member.



ICC-ES Evaluation Report ESR-2994

Reissued August 2021
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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES
Section: 06 17 33—Wood Joists

REPORT HOLDER: REDBUILT LLC

EVALUATION SUBJECT: RED-1™ PREFABRICATED WOOD I-JOISTS

1.0 EVALUATION SCOPE
 Compliance with the following codes:
 ■ 2018, 2015, 2012 and 2009 International Building Code® (IBC)
 ■ 2018, 2015, 2012 and 2009 International Residential Code® (IRC)
 For evaluation for compliance with codes adopted by the Los Angeles Department of Building and Safety (LADBS), see ESR-2994 LABC and LABC Supplement.

Properties evaluated:
 ■ Structural
 ■ Sound ratings
 ■ Fire-resistance ratings

2.0 USES
 Red-1 joists are prefabricated wood I-joists used as floor joists, roof rafters and blocking panels, to support code-required loads. Red-1 joists described in Table 1 are also used as rim joists, to provide the transfer of vertical loads at the rim joist location, diaphragm attachment, and transfer of lateral loads. Prefabricated wood I-joists described in this report comply with Section 2303.1.2 and 2303.1.3 of the IRC, for allowable stress design, and Section R502.1.2 and R502.1.7 of the IRC (Section R502.1.4 of the 2012 and 2009 IRC).

3.0 DESCRIPTION
 Red-1 joists are prefabricated wood I-joists having wood or wood-based flanges and oriented strand board (OSB) webs. Either the top and bottom flanges are parallel, forming a constant-depth joist (non-tapered), or the web at the top flange location has a single taper, forming a variable-depth joist (tapered). Red-145, -165, and -180 are available as tapered and non-tapered I-joists. The remaining I-joists are non-tapered. The web-to-web connection is either built vertically, and the web-to-web connection is either built

jointed or serrated and glued to form a continuous web. The web-to-flange connection is a proprietary tongue-and-groove joint. Refer to Table 1 for Red-1 joist series and material descriptions.

3.1 General:
 Red-1 joists are prefabricated wood I-joists having wood or wood-based flanges and oriented strand board (OSB) webs. Either the top and bottom flanges are parallel, forming a constant-depth joist (non-tapered), or the web at the top flange location has a single taper, forming a variable-depth joist (tapered). Red-145, -165, and -180 are available as tapered and non-tapered I-joists. The remaining I-joists are non-tapered. The web-to-web connection is either built vertically, and the web-to-web connection is either built

jointed or serrated and glued to form a continuous web. The web-to-flange connection is a proprietary tongue-and-groove joint. Refer to Table 1 for Red-1 joist series and material descriptions.

3.2 Flanges: Flange material is RedLam™ laminated veneer lumber (LVL) that meets the requirements noted in the approved quality documentation that contains RedBUILT™ manufacturing standards. The assigned allowable tension stresses for RedLam™ LVL flanges are verified in accordance with the procedures set forth in the approved quality documentation. Table 1 of this report specifies flange material, widths and depths. Flange material and grades are as specified in the quality documentation that contains RedBUILT™ manufacturing standards.

3.2.2 Webs: Web material is OSB conforming to DDC Voluntary Product Standard P52, Exposure 1, along with further requirements set forth in the quality documentation that contains RedBUILT™ manufacturing standards. Web material thickness requirements are noted in Table 1 of this report.

3.2.3 Adhesives: Adhesives are of the types specified in the quality documentation that contains RedBUILT™ manufacturing standards.

4.0 DESIGN AND INSTALLATION
4.1 General:
 The design and installation of Red-1 joists described in this report must comply with Sections 4.2 through 4.16. Additionally, design of Red-1 joists is governed by the applicable code and corresponding editions of ANSI/AWC National Design Specification® for Wood Construction (NDS).

4.2 Design Values:
 Table 2 specifies reference design moments, reactions, vertical shear forces, and joist stiffness (EI). Reference design reactions are based on minimum bearing lengths of 1 1/2 inches, 2 1/2 inches and 3 1/2 inches (45, 64 and 89 mm), for simple spans; and 3 1/2, 5 1/2 and 7 inches (89, 133 and 178 mm) at intermediate support points for continuous spans. Tapered joists and custom depth rafter flange joists are designed in accordance with the approved quality documentation and are provided by RedBUILT. When flange joists are used as multiple span members, and subject to uniform load only, the calculated shear force used for design at the intermediate support may be reduced by the percentage determined from the following formula and limited to the depths shown in Table 3.

3.1 General:
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jointed or serrated and glued to form a continuous web. The web-to-flange connection is a proprietary tongue-and-groove joint. Refer to Table 1 for Red-1 joist series and material descriptions.

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ESR-2994 | Most Widely Accepted and Trusted | Page 8 of 16

RED-1 JOIST SERIES	FLANGE SIZE, DEPTH x WIDTH (inches)	FLANGE MATERIAL	WEB THICKNESS (inches)	RANGE OF JOIST DEPTHS (inches)
Red-145L	1.375 (minimum) x 1.75	LVL	1/2	9 1/2 - 14
Red-153	1.375 (minimum) x 2.1	LVL	1/2	9 1/2 - 16
Red-158	1.375 (minimum) x 2.3	LVL	1/2	9 1/2 - 16
Red-145	1.5 x 1.75	LVL	1/2	7 1/2 - 20 (8 - 20 taper)
Red-165	1.5 (minimum) x 2.5	LVL	1/2	7 1/2 - 30 (7 1/2 - 30 taper)
Red-190	1.5 (minimum) x 3.5	LVL	1/2	11 1/2 - 30 (9 1/2 - 30 taper)
Red-190H	1.75 x 3.5	LVL	1/2	11 1/2 - 30
Red-190HD	2.125 x 3.5	LVL	1/2	11 1/2 - 32
Red-190HS	2.5 x 3.5	LVL	1/2	11 1/2 - 32

For S: 1 inch = 25.4 mm. For D: 1 inch = 25.4 mm.

TABLE 2—REFERENCE DESIGN VALUES FOR RED-1 JOISTS DESCRIBED IN TABLE 1^{1,2,3}

JOIST DEPTH (in.)	JOIST WEIGHT (plf)	Moment M (ft.-lbs./ft.)	Shear V (lbs.)	EI (10 ⁶ lbs.-ft. ²)	REFERENCE DESIGN VALUES											
					END REACTION, R _e (lbs.) ^{1,2,3}						INTERMEDIATE REACTION, R _i (lbs.) ^{1,2,3}					
					2 1/2" ^m	3 1/2"	4 1/2"	5 1/2" ⁿ	6 1/2"	7 1/2"	8 1/2"	9 1/2"	10 1/2"	11 1/2"	12 1/2"	
Red-145																
7 1/2	2.0	2720	860	775	103	4.5	860	NA	860	NA	2025	NA	2025	NA	NA	NA

TABLE 2304.10.1 - continued
FASTENING SCHEDULE

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
1. Blocking between ceiling joists, rafters or trusses to top plate or other framing below	3-8d common (2 1/2" x 0.1314) or 3-10d box (3/8" x 0.1289) or 3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	Each end, toenail
Blocking between rafters or truss not at the wall top plate, to rafter or truss	2-8d common (2 1/2" x 0.1314) 2-3/8" x 0.1316 nails 2-3/4" gage staples, 7/166crown	Each end, toenail
Flat blocking to truss and web filler	1-6d common (3 1/2" x 0.1629) @ 660 c. 3/8" x 0.1316 nails @ 660 c. 3/8" x 14 gage staples @ 660 c.	Face nail
2. Ceiling joists to top plate	3-8d common (2 1/2" x 0.1314) or 3-10d box (3/8" x 0.1289) or 3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	Each joist, toenail
3. Ceiling joist not attached to parallel rafter, laps over partitions (no thru)	3-16d common (3 1/2" x 0.1629) or 4-10d box (3/8" x 0.1289) or 4-3/8" x 0.1316 nails, or 4-3/4" gage staples, 7/166crown	Face nail
4. Ceiling joist attached to parallel rafter (heel joint)	Per Table 2308.7.3.1	Face nail
5. Collar tie to rafter	3-10d common (3/8" x 0.1289) or 4-10d box (3/8" x 0.1289) or 4-3/8" x 0.1316 nails, or 4-3/4" gage staples, 7/166crown	Face nail
6. Rafter or roof truss to top plate	3-10d common (3/8" x 0.1289) or 3-16d box (3/8" x 0.1289) or 4-10d box (3/8" x 0.1289) or 4-3/8" x 0.1316 nails, or 4-3/4" gage staples, 7/166crown	Toenail
7. Roof rafters to ridge valley or hip rafters, or roof rafter to 2-inch ridge beam	2-16d common (3 1/2" x 0.1629) or 3-10d box (3/8" x 0.1289) or 3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	End nail
	3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	Toenail

(continued)

TABLE 2304.10.1 - continued
FASTENING SCHEDULE

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
8. Stud to stud (not at braced wall panels)	1-6d common (3 1/2" x 0.1629) 240 c. face nail 1-10d box (3/8" x 0.1289) or 3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	1660 c. face nail
9. Stud to stud and abutting studs at intersecting wall corners (at braced wall panels)	1-6d common (3 1/2" x 0.1629) or 1-6d box (3 1/2" x 0.1354) or 3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	1660 c. face nail 1260 c. face nail
10. Built-up header (2x6 to 2xheader)	1-6d common (3 1/2" x 0.1629) or 1-6d box (3 1/2" x 0.1354)	1660 c. each edge, face nail 1260 c. each edge, face nail
11. Continuous header to stud	4-8d common (2 1/2" x 0.1314) or 4-10d box (3/8" x 0.1289)	Toenail
12. Top plate to top plate	1-10d box (3/8" x 0.1289) or 3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	1260 c. face nail
13. Top plate to top plate, at end joints	8-16d common (3 1/2" x 0.1629) or 12-10d box (3/8" x 0.1289) or 12-3/8" x 0.1316 nails, or 12-3/4" gage staples, 7/166crown	Each side of end joint, face nail (minimum 24" lap splice length each side of end joint)
14. Bottom plate to joist, rim joist, band joist or blocking (not at braced wall panels)	1-6d common (3 1/2" x 0.1629) or 1-6d box (3 1/2" x 0.1354) or 3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	1260 c. face nail
15. Bottom plate to joist, rim joist, band joist or blocking at braced wall panels	2-16d common (3 1/2" x 0.1629) or 3-16d box (3 1/2" x 0.1354) or 4-3/8" x 0.1316 nails, or 4-3/4" gage staples, 7/166crown	1660 c. face nail
16. Stud to top or bottom plate	4-8d common (2 1/2" x 0.1314) or 4-10d box (3/8" x 0.1289) or 4-3/8" x 0.1316 nails, or 4-3/4" gage staples, 7/166crown	Toenail
	2-16d common (3 1/2" x 0.1629) or 3-10d box (3/8" x 0.1289) or 3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	End nail
17. Top plates, laps at corners and intersections	2-16d common (3 1/2" x 0.1629) or 3-10d box (3/8" x 0.1289) or 3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	Face nail

TABLE 2304.10.1 - continued
FASTENING SCHEDULE

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
18. 16-brace to each stud and plate	2-8d common (2 1/2" x 0.1314) or 2-10d box (3/8" x 0.1289) or 2-3/8" x 0.1316 nails, or 2-3/4" gage staples, 7/166crown	Face nail
19. 16-66sheathing to each bearing	2-8d common (2 1/2" x 0.1314) or 2-10d box (3/8" x 0.1289)	Face nail
20. 16" 66and wider sheathing to each bearing	3-8d common (2 1/2" x 0.1314) or 3-10d box (3/8" x 0.1289)	Face nail
21. Joist to sill, top plate, or girder	3-8d common (2 1/2" x 0.1314) or floor 3-10d box (3/8" x 0.1289) or 3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	Toenail
22. Rim joist, band joist, or blocking to top plate, sill or other framing below	8d common (2 1/2" x 0.1314) or 10d box (3/8" x 0.1289) or 3/8" x 0.1316 nails, or 3/4" gage staples, 7/166crown	660 c., toenail
23. 16" 66subfloor or less to each joist	2-8d common (2 1/2" x 0.1314) or 2-10d box (3/8" x 0.1289)	Face nail
24. 26subfloor to joist or girder	2-16d common (3 1/2" x 0.1629)	Face nail
25. 26planks (joist & beam - floor & roof)	2-16d common (3 1/2" x 0.1629)	Each bearing, face nail
26. Built-up girders and beams, 26lumber layers	20d common (4/8" x 0.1929) 10d box (3/8" x 0.1289) or 3/8" x 0.1316 nails, or 3/4" gage staples, 7/166crown	32" o.c., face nail at top and bottom staggered on opposite sides 24" o.c., face nail at top and bottom staggered on opposite sides
	2-20d common (4/8" x 0.1929) or 3-10d box (3/8" x 0.1289) or 3-3/8" x 0.1316 nails, or 3-3/4" gage staples, 7/166crown	Ends and at each splice, face nail
27. Ledger strip supporting joists or rafters	3-16d common (3 1/2" x 0.1629) or 4-10d box (3/8" x 0.1289) or 4-3/8" x 0.1316 nails, or 4-3/4" gage staples, 7/166crown	Each joist or rafter, face nail
28. Joist to band joist or rim joist	3-16d common (3 1/2" x 0.1629) or 4-10d box (3/8" x 0.1289) or 4-3/8" x 0.1316 nails, or 4-3/4" gage staples, 7/166crown	End nail
29. Bridging or blocking to joist, rafter or truss	2-8d common (2 1/2" x 0.1314) or 2-10d box (3/8" x 0.1289) or 2-3/8" x 0.1316 nails, or 2-3/4" gage staples, 7/166crown	Each end, toenail

TABLE 2304.10.1 - continued
FASTENING SCHEDULE

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
Wood structural panels (WSP), subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing	6d common or deformed (2 1/2" x 0.1139) (subfloor and wall)	6" 12"
	8d common or deformed (2 1/2" x 0.1314) (roof) or RSRS-01 (2 3/8" x 0.1139) nail (roof)	6" 12"
30. 3/8" - 1/2"	2 3/8" x 0.1136 nail (subfloor and wall)	6" 12"
	1 3/4" x 16 gage staple, 7/166crown (subfloor and wall)	4" 8"
	2 3/8" x 0.1136 nail (roof)	4" 8"
	1 3/4" x 16 gage staple, 7/166crown (roof)	3" 6"
31. 1/2" - 3/4"	8d common (2 1/2" x 0.1314) or 6d deformed (2 1/2" x 0.1139) (subfloor and wall)	6" 12"
	8d common or deformed (2 1/2" x 0.1314) (roof) or RSRS-01 (2 3/8" x 0.1139) nail (roof)	6" 12"
	2 3/8" x 0.1136 nail or 2 1/2" x 16 gage staple, 7/166crown	4" 8"
32. 1/2" - 1 1/4"	10d common (3/8" x 0.1289) or 8d deformed (2 1/2" x 0.1139)	6" 12"
Other exterior wall sheathing		
33. 1/2" 66fiberboard sheathing ^b	1 1/2" galvanized roofing nail 7/166d diameter, or 1 1/4" x 16 gage staple with 7/166d 16crown	3" 6"
34. 3/8" 66fiberboard sheathing ^b	1 3/4" galvanized roofing nail 7/166d diameter head, or 1 1/2" x 16 gage staple with 7/166d 16crown	3" 6"
Wood structural panels, combination subfloor underlayment to framing		
35. 3/8" and less	8d common (2 1/2" x 0.1314) or 6d deformed (2 1/2" x 0.1139)	6" 12"
36. 3/8" - 1/2"	8d common (2 1/2" x 0.1314) or 8d deformed (2 1/2" x 0.1139)	6" 12"
37. 1/2" - 1 1/4"	10d common (3/8" x 0.1289) or 8d deformed (2 1/2" x 0.1139)	6" 12"
Faced siding to framing		
38. 1/2" or less	6d corrosion-resistant siding (1 7/8" x 0.1064) or 6d corrosion-resistant casing (2 1/2" x 0.0996)	6" 12"
39. 3/4"	8d corrosion-resistant siding (2 3/8" x 0.1289) or 8d corrosion-resistant casing (2 1/2" x 0.1139)	6" 12"

TABLE 2304.10.1 - continued
FASTENING SCHEDULE

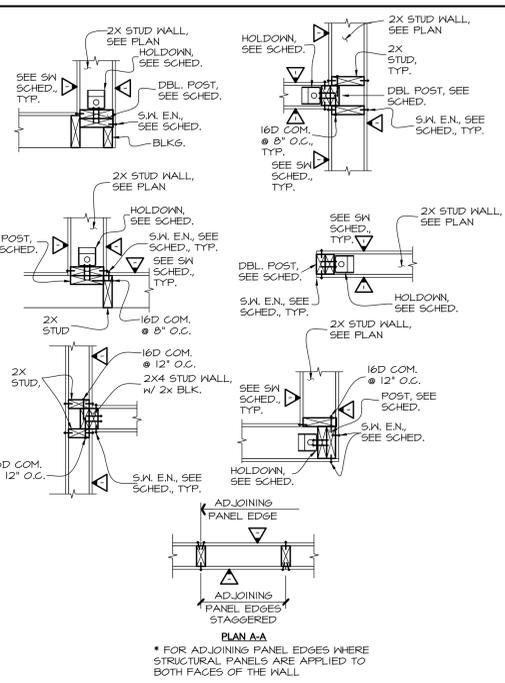
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
Wood structural panels (WSP), subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing		
	interior panels	
40. 1/2"	4d casing (1 1/2" x 0.0809) or 4d finish (1 1/2" x 0.0729)	6" 12"
41. 3/4"	6d casing (2 1/2" x 0.0996) or 6d finish (Panel supports at 24 inches)	6" 12"

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DATE	ISSUED FOR	ENGEL & COMPANY <i>Engineers</i> 4009 UNION AVENUE BAKERSFIELD, CA 93305 www.engelengineers.com (661) 327-7025	DRAWN: ETC DATE: 10/20/2021 CHECKED: JH APPROVED:	Structural Notes Shopping Center Rancho Vista Shopping Center 3007 Rancho Vista Blvd. Palmdale, CA 93551	SHEET NO. S02 OF 23
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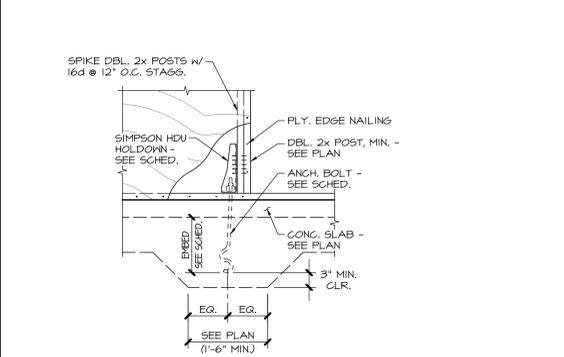
10/21/2021



SHEAR WALL SCHEDULE						
MARK	SHEATHING ^{1,2}	NAILING ⁴	SILL BOLTS ⁵ (IN NEW CONCRETE)	RETROFIT SILL BOLTS ⁵ (IN EXISTING CONCRETE)	SHEAR TRANSFER	DETAIL
▲	1/2" STRUCTURAL I (BLOCKED)	10d COM @ 6" O.C. EDGES, 12" O.C. FIELD.	3/8"x12" @ 48" O.C.	3/8"x12" EMBED, TITEN HD ⁷ @ 48" O.C.	SIMPSON A35 @ 24" O.C.	2x N/A
▲	1/2" STRUCTURAL I (BLOCKED)	10d COM @ 6" O.C. EDGES, 12" O.C. FIELD. (TWO SIDED)	3/8"x12" @ 24" O.C.	3/8"x12" EMBED, TITEN HD ⁷ @ 24" O.C.	SIMPSON A35 @ 16" O.C.	3x OR 2x W/ OFFSET PANELS. PLAN A-A (SEE ABOVE)
▲	1/2" STRUCTURAL I (BLOCKED)	10d COM @ 4" O.C. EDGES, 12" O.C. FIELD. (TWO SIDED)	3/8"x12" @ 16" O.C.	3/8"x12" EMBED, TITEN HD ⁷ @ 16" O.C.	SIMPSON A35 @ 16" O.C.	3x N/A

- SHEAR WALL NOTES**
- WALL SHEATHING ON SHEAR WALLS SHALL BE CONTINUOUS FROM BOTTOM OF SILL PLATE TO TOP OF DOUBLE TOP PLATE, TYPICALLY, UNLESS NOTED OTHERWISE.
 - WHERE WALL SHEATHING IS SHOWN ON ONE SIDE IT MAY BE PLACED ON OTHER SIDE OF WALL (COORDINATE WITH ARCHITECTURAL).
 - SEE GENERAL NOTES FOR SHEATHING.
 - ATTACH SHEATHING WITH FASTENERS AND SPACING AS SHOWN AT PANEL EDGES. AT INTERMEDIATE FRAMING USE FASTENER SIZE AS SHOWN AND A SPACING OF 12" ON CENTER. BLOCK ALL UNSUPPORTED EDGES. USE COMMON NAILS IN PLYWOOD SHEATHING.
 - ANCHOR BOLTS FOR THE PRESSURE TREATED SILL PLATE SHALL BE 10" LONG, TYPICALLY, UNLESS NOTED OTHERWISE. ALL SILL BOLTS SHALL HAVE 3" x 3" x 22# MIN. THICK PLATE WASHERS.
 - ANCHOR BOLTS FOR THE PRESSURE TREATED SILL PLATE SHALL BE 10" LONG, TYPICALLY, UNLESS NOTED OTHERWISE. ALL SILL BOLTS SHALL HAVE 3" x 3" x 22# MIN. THICK PLATE WASHERS.
 - SILL SCREWS SHALL BE SIMPSON "SECN" WITH LENGTH AS SHOWN.
 - WHERE 2x OR 3x PLATE AND PLYWOOD PANEL SPLICES ARE INDICATED, FOUNDATION SILL PLATES AND ALL FRAMING MEMBERS RECEIVING EDGE FASTENING FROM ADJUTING PANELS (i.e. PANEL SPLICES) SHALL NOT BE SMALLER THAN A SINGLE FRAMING MEMBER OF THE NOMINAL SIZE SHOWN.
 - *TITEN HD⁷ NOTE: SHALL "X" (DIMED) DEEP HOLE AND PREPARE PER SIMPSON SPECIFICATIONS. INSTALL SIMPSON "TITEN HD" PER SIMPSON SPECIFICATIONS. DO NOT EXCEED INSULATION THICKNESS.

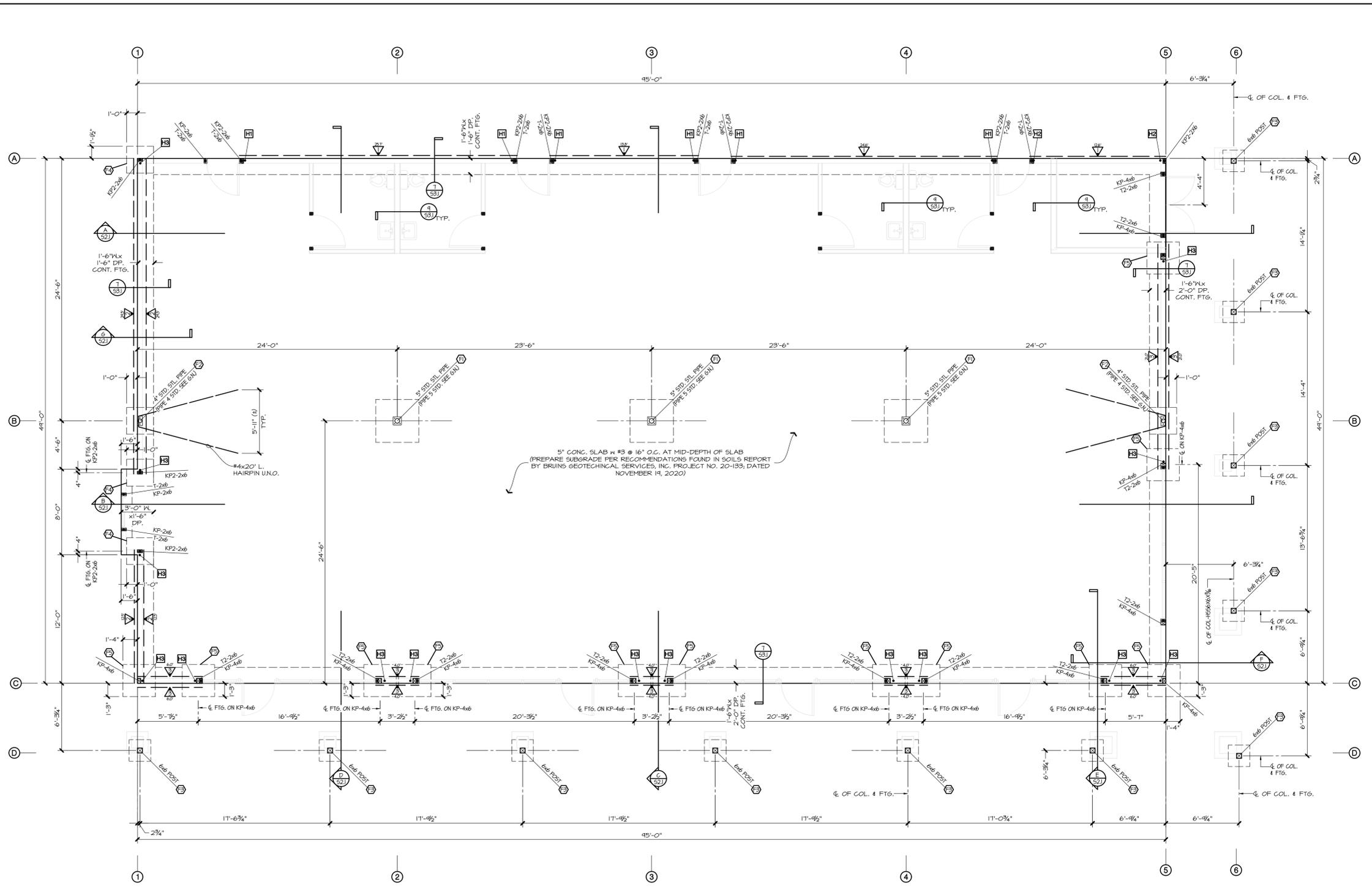
1 SHEAR WALL SCHEDULE DETAIL N.T.S.



HOLDOWN SCHEDULE			
MARK	HOLDOWN ^{1,2}	ATTACH. TO POST ³	ANCHOR BOLT ^{4,5}
H1	HDU2-SD525	(6) SD9X28	SIMP. 56TB6
H2	HDU4-SD525	(10) SD9X28	SIMP. 56TB20
H3	HDU11-SD525	(30) SD9X28	SIMP. PAB8-24

- HDU-TYPE HOLDOWNS ARE THOSE MANUFACTURED BY SIMPSON STRONG TIE (ICC ESR-2330)
- HD-TYPE HOLDOWNS ARE THOSE MANUFACTURER BY SIMPSON STRONG TIE (ER-143)
- SDS-TYPE SCREWS ARE THOSE MANUFACTURED BY SIMPSON STRONG TIE (ICC ESR-2236)
- 56TB-TYPE ANCHOR BOLTS ARE THOSE MANUFACTURED BY SIMPSON STRONG TIE (ICC ESR-2611)
- SEE PLAN FOR MINIMUM FOUNDATION DEPTH INTO NATURAL GRADE. THICKEN FOUNDATION AS REQUIRED TO PROVIDE 3" MINIMUM CONCRETE COVER BELOW ANCHOR.

2 HOLDOWN SCHEDULE DETAIL N.T.S.



FOUNDATION PLAN

- FOR BUILDING LOCATION AND ORIENTATION, SEE SITE PLAN BY MCLERNON ARCHITECTURE GROUP
- SEE FOOTING SCHEDULE ON SHEET
- SEE HOLDOWN SCHEDULE ON SHEET
- SEE SHEAR WALL SCHEDULE ON SHEET

FOOTING SCHEDULE ¹							
MARK	SIZE	DEPTH	ANCHORAGE		REINFORCEMENT ⁴		DETAIL
			ANCHOR BOLTS ²	EMBEDMENT ³	BOTTOM REINFORCEMENT	TOP REINFORCEMENT	
F1	4'-0" SQ.	2'-6"	(4) 3/4" ASTM F1554 GR. 36 THRO'D ROD ANCHORS W/ DEL. HEX NUT @ BOT.	18" EMBED	(4) #4 GR. 40 EA. WAY	(4) #4 GR. 40 EA. WAY	1 (S1)
F2	2'-6" SQ.	2'-0"	(2) 3/4" ASTM F1554 GR. 36 THRO'D ROD ANCHORS W/ DEL. HEX NUT @ BOT.	18" EMBED	(3) #4 GR. 40 EA. WAY	(3) #4 GR. 40 EA. WAY	2 (S1)
F3	2'-0" SQ.	2'-0"	-	-	(2) #4 GR. 40 EA. WAY	(2) #4 GR. 40 EA. WAY	3 (S1)
F4	2'-6" SQ.	2'-6"	-	-	(3) #4 GR. 40 EA. WAY	(3) #4 GR. 40 EA. WAY	2 (S1)
F5	3'-0" SQ.	3'-3"	-	-	(3) #4 GR. 40 EA. WAY	(3) #4 GR. 40 EA. WAY	2 (S1)

- FOR FOOTING CALLOUT AND PLACEMENT, SEE FOUNDATION PLAN.
- ANCHOR BOLT SPECIFICATION SHALL BE ASTM F1554 GRADE 36, U.N.O. ON SCHEDULE.
- EMBEDMENT IS TO BE FOLLOWED STRICTLY, SEE RESPECTIVE FOOTING DETAIL.

FRAMING CALLOUT LEGEND	
MEMBER QUANTITY - IF OMITTED USE ONE	MEMBER SIZE
T-2x6	
MEMBER TYPE	
T: TRIMMER	
K: KING	
P: POST	
COL: COLUMN	

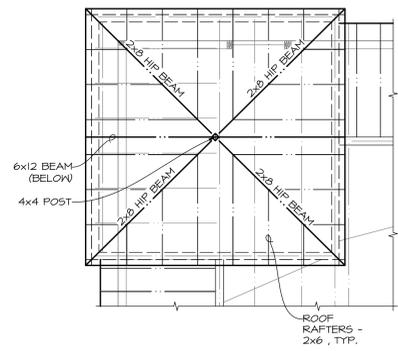
WALL LEGEND	
- 2x6 STUDS @ 16" O.C.	
- 2x4 STUDS @ 16" O.C.	
- 1 HOUR FIREWALL - 2x6 STUDS @ 16" O.C. 3/4" TYPE "X" GYP. BRD. BOTH SIDES EXTEND TO ROOF SHT'G. SEE 1 (S1)	

1/4" = 1'-0"

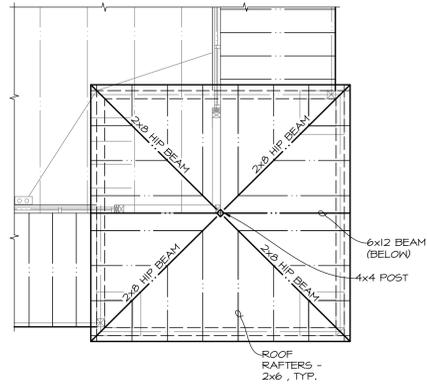


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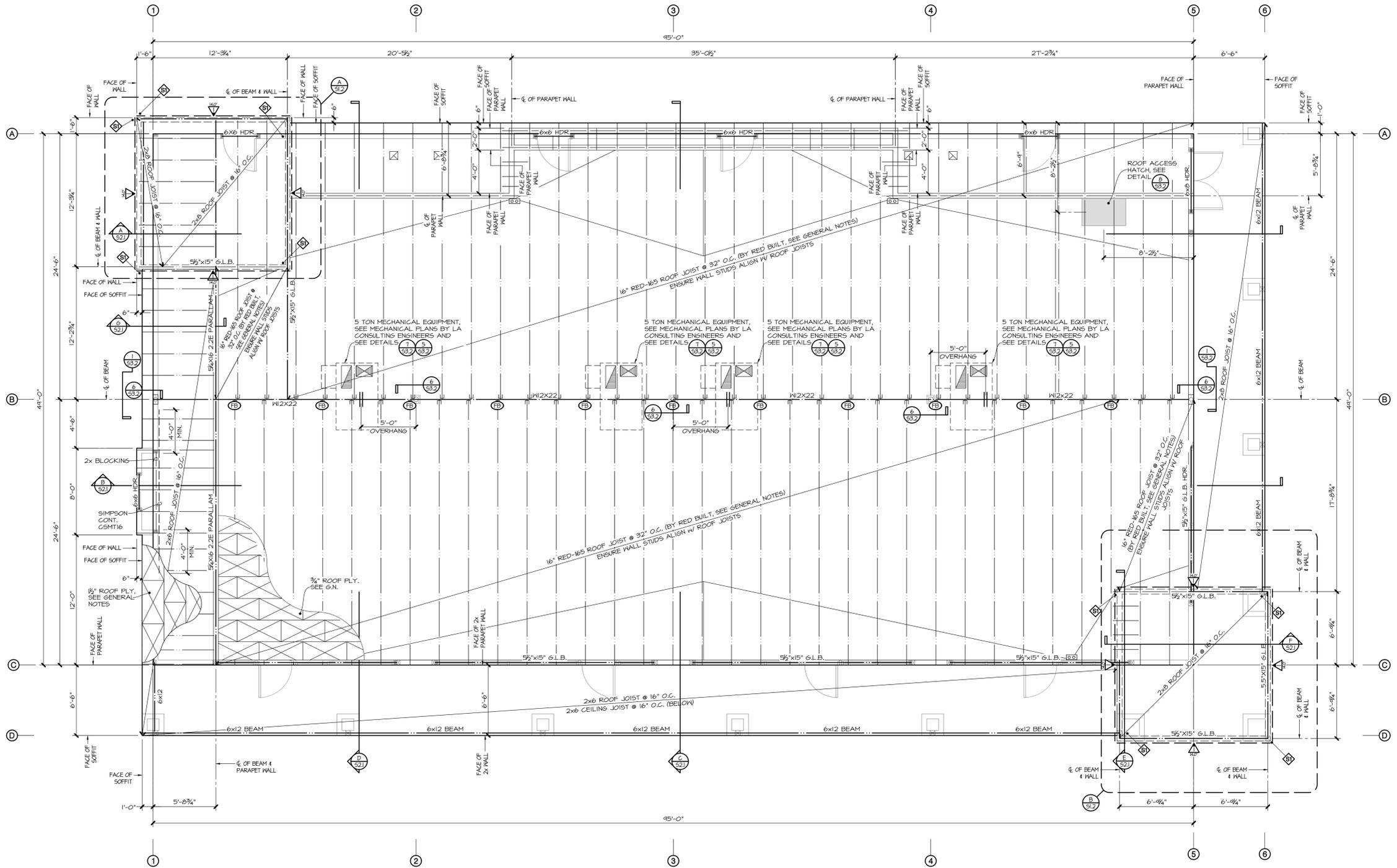
ENGEL & COMPANY Engineers 4009 UNION AVENUE BAKERSFIELD, CA 93305 www.engeengineers.com (661) 327-7025		DRAWN: ECH DATE: CHECKED: LH APPROVED: NO. S-2710 EXP. 6/30/22 STRUCTURAL STATE OF CALIFORNIA	Foundation Plan Shopping Center Rancho Vista Shopping Center 3007 Rancho Vista Blvd. Palmdale, CA 93551	SHEET NO. S11 OF
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A) NORTH PARTIAL ROOF FRAMING PLAN 1/4" = 1'-0"



B) SOUTH PARTIAL ROOF FRAMING PLAN 1/4" = 1'-0"

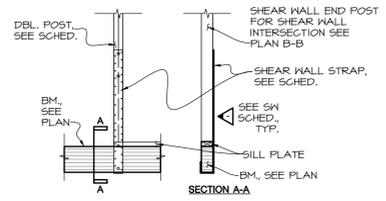
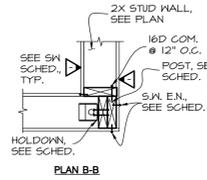


ROOF FRAMING PLAN

- Ⓛ - DENOTES FLANGE BRACE @ 8'-0" O.C. MAX. - SEE DETAIL 10/337
- ◇ - DENOTES SHEAR WALL STRAP SCHEDULE - SEE DETAIL 10/337
- U - DENOTES SIMPSON ITS2.56/16 HANGER W/ WEB STIFFENERS REQUIRED PER DETAIL 4/377 FOR DOUBLE RED-165 JOISTS, USE SIMPSON WF4.28x116
- || - DENOTES BEAM SPLICE/HINGE, SEE DETAIL 6/337

SHEARWALL STRAP SCHEDULE	
STRAP 1	
M5C40	
M5C52	

1. M5C STRAPS ARE THOSE MANUFACTURED BY SIMPSON STRONG TIE



1) SHEAR WALL STRAP SCHEDULE DETAIL

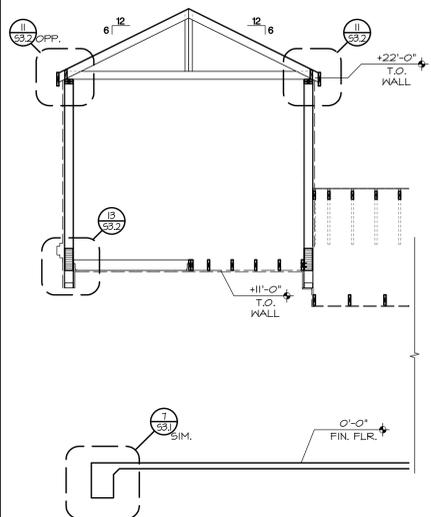
N.T.S.

1/4" = 1'-0"

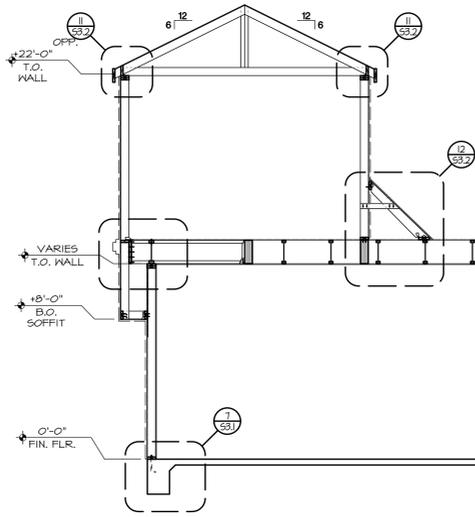


10/21/2021

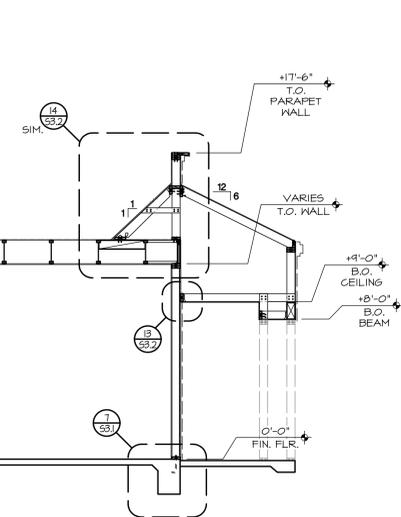
ENGEL & COMPANY Engineers 4009 UNION AVENUE BAKERSFIELD, CA 93305 www.engelengineers.com (661) 327-7025		DRAWN: EMT DATE: 10/20/2021 CHECKED: JLV APPROVED:	Roof Framing Plan Shopping Center Rancho Vista Shopping Center 3007 Rancho Vista Blvd. Palmdale, CA 93551	SHEET NO: S12 OF 234/235
DATE	ISSUED FOR			



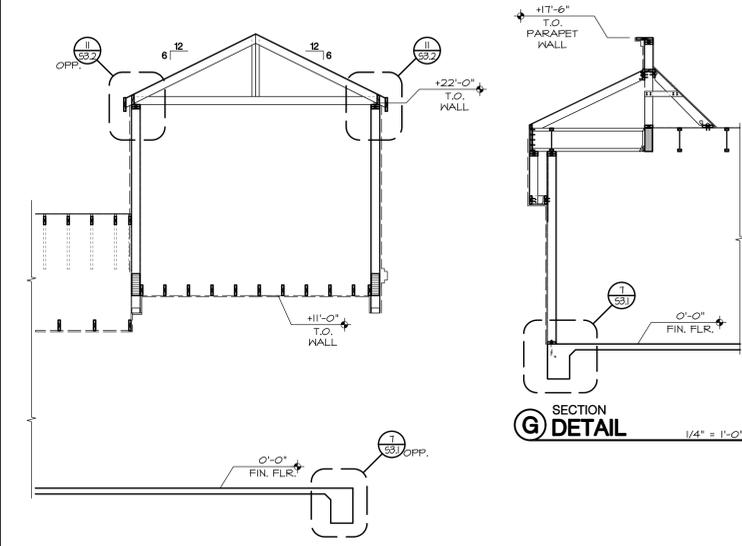
E SECTION 1/4" = 1'-0"



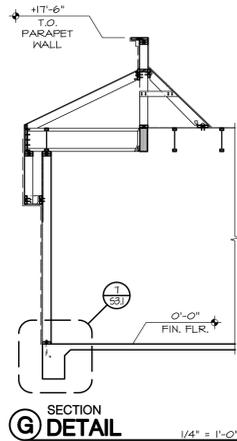
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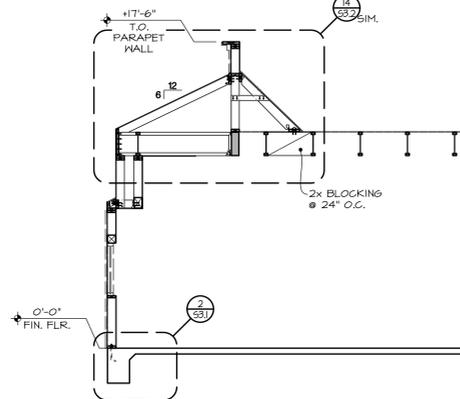
1/4" = 1'-0"



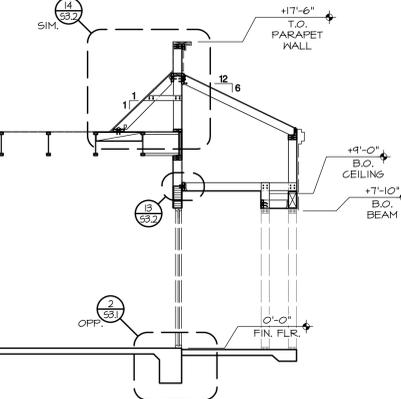
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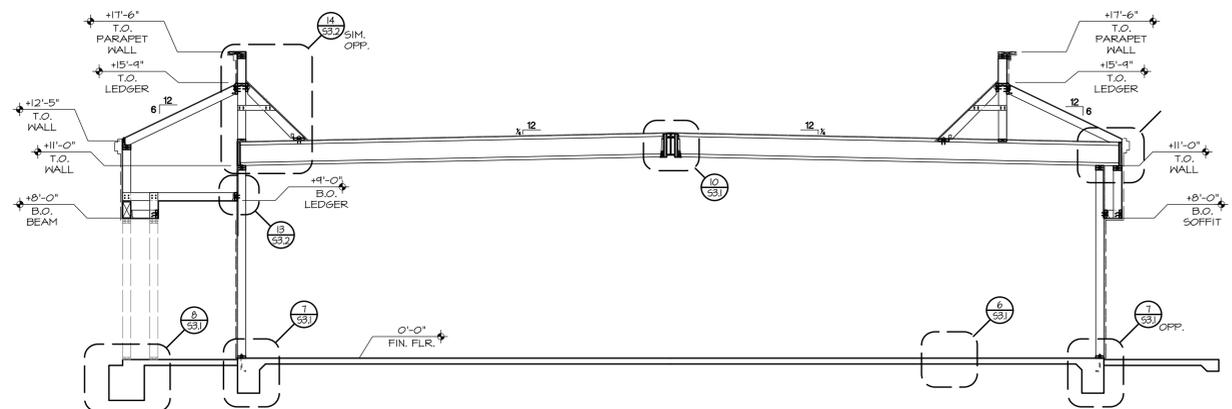
G SECTION DETAIL 1/4" = 1'-0"



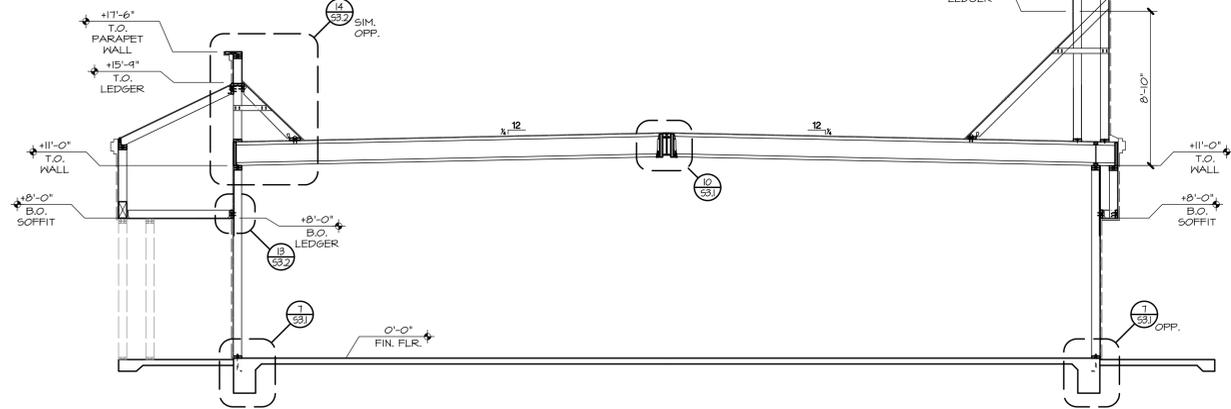
B SECTION 1/4" = 1'-0"



1/4" = 1'-0"



D SECTION 1/4" = 1'-0"



C SECTION 1/4" = 1'-0"

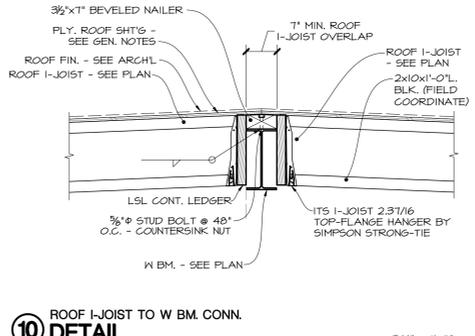
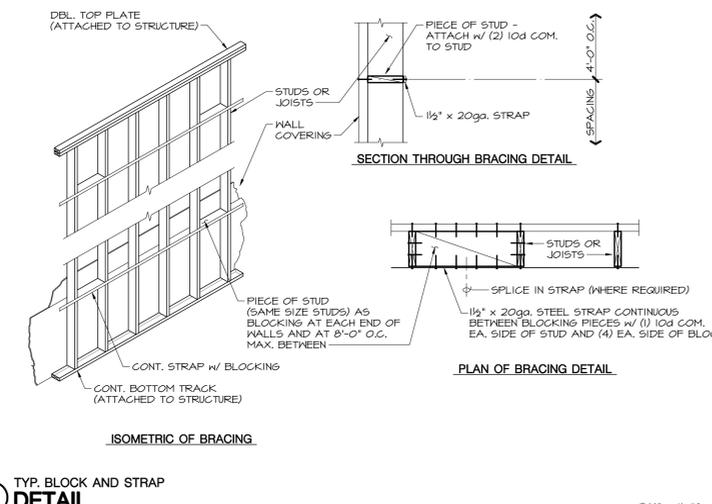
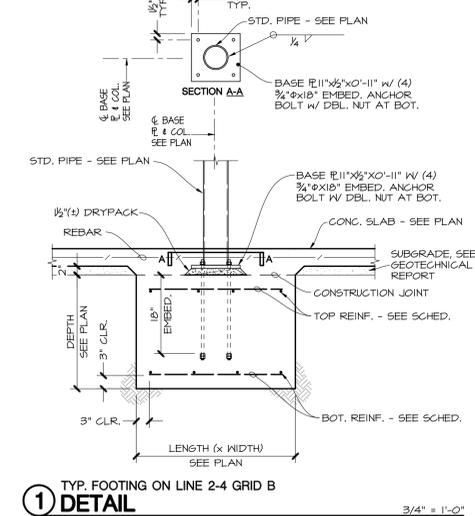
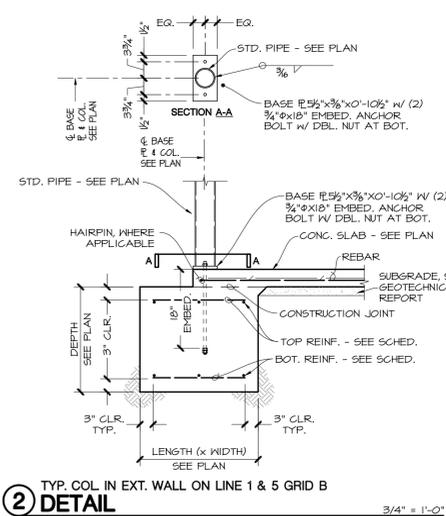
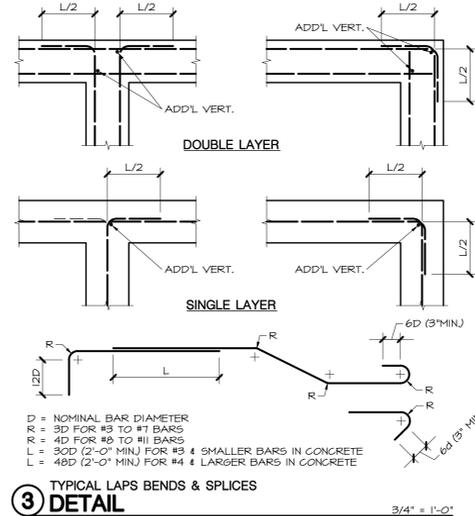
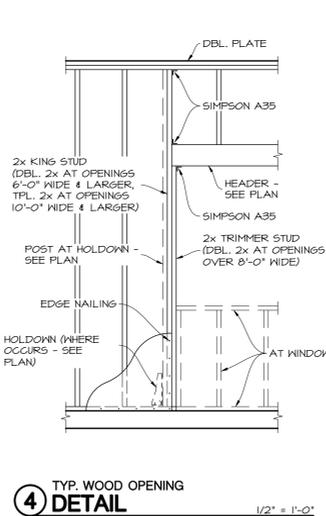
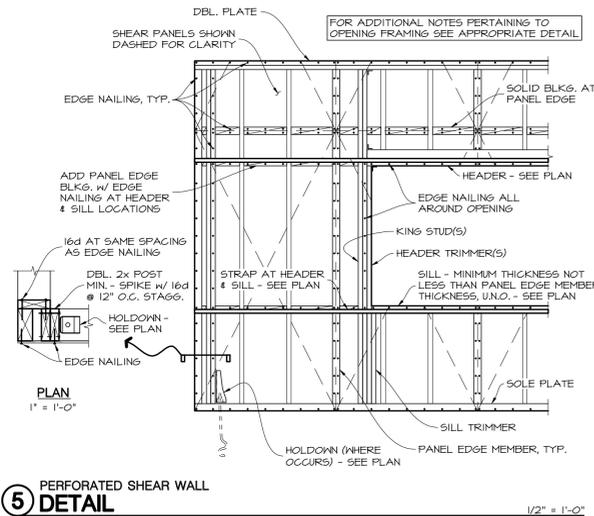
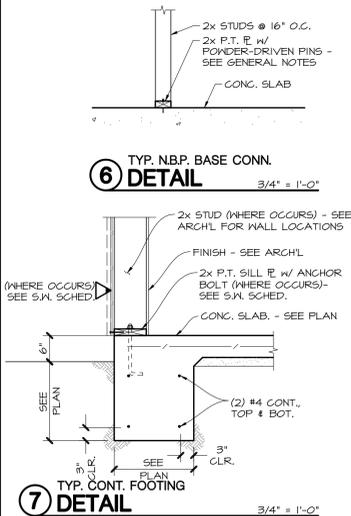
1/4" = 1'-0"

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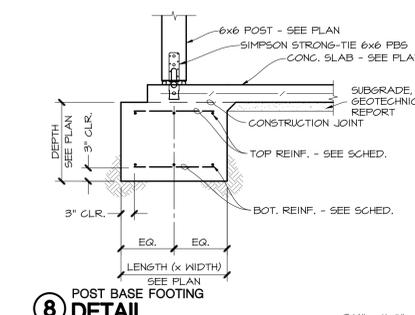


10/21/2021

		ENGEL & COMPANY <i>Engineers</i> 4009 UNION AVENUE BAKERSFIELD, CA 93305 www.engelengineers.com (661) 327-7025	DRAWN EHM	Sections Shopping Center Rancho Vista Shopping Center 3007 Rancho Vista Blvd. Palmdale, CA 93551		SHEET NO. S21
DATE ISSUED FOR			CHECKED JHM	DATE 10/20/2021		

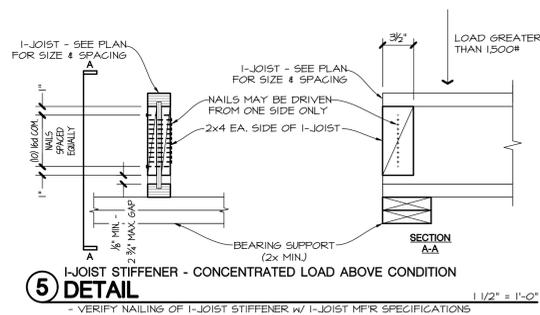


GA FILE NO. WP 3341	GENERIC	1 HOUR FIRE	45 TO 49 STC SOUND
GYPSON WALLBOARD, WOOD STUDS Base layer 1/4" gypsum wallboard applied parallel to each side of 2 x 6 wood studs 16" o.c. with 4d coated nails, 1-1/2" long, 0.009" shank, 1/4" heads, 12" o.c. studs staggered 1/2" on opposite sides. Face layer 1/2" type X plain or predecorated gypsum wallboard or gypsum veneer base applied parallel to each side with 1/4" beads of adhesive 2" o.c. and 6d coated nails, 1-7/8" long, 0.0915" shank, 1/4" heads, 6" o.c. at top and bottom plates only. Offset joints 24" from base layer joints. (LOAD-BEARING)			
		Thickness:	5-1/8"
		Approx. Weight:	7 pcf
		Fire Test:	FM WP-147, 1-2-69
		Sound Test:	NGC 2321, 8-29-68

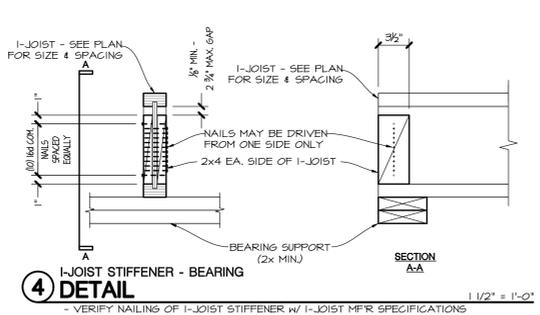


ENGEL & COMPANY <i>Engineers</i> 4009 UNION AVENUE BAKERSFIELD, CA 93305 www.engeengineers.com (661) 327-7025		DRAWN: E.H. DATE: 10/20/2021 CHECKED: J.H. APPROVED:	Details Shopping Center Rancho Vista Shopping Center 3007 Rancho Vista Blvd. Palmdale, CA 93551	SHEET NO. S3.1 OF
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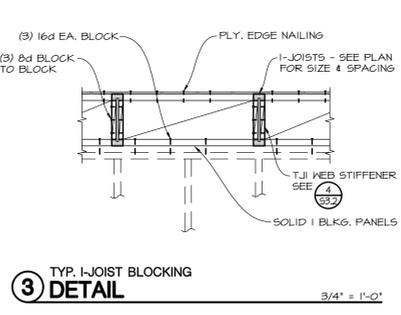
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 Plot: 10/20/2021 10:47 AM
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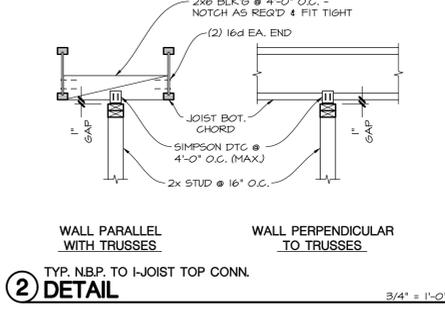
5 I-JOIST STIFFENER - CONCENTRATED LOAD ABOVE CONDITION
1 1/2" = 1'-0"
- VERIFY NAILING OF I-JOIST STIFFENER W/ I-JOIST MFR SPECIFICATIONS



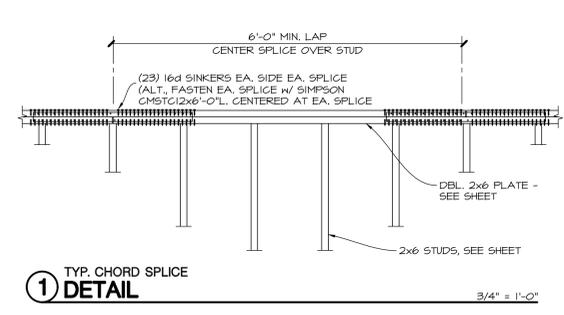
4 I-JOIST STIFFENER - BEARING
1 1/2" = 1'-0"
- VERIFY NAILING OF I-JOIST STIFFENER W/ I-JOIST MFR SPECIFICATIONS



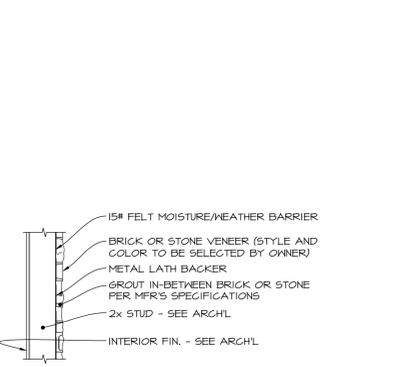
3 TYP. I-JOIST BLOCKING
3/4" = 1'-0"



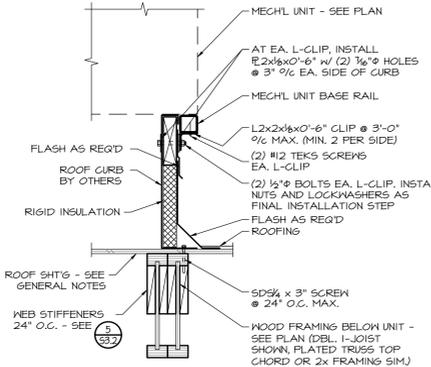
2 TYP. N.B.P. TO I-JOIST TOP CONN.
3/4" = 1'-0"



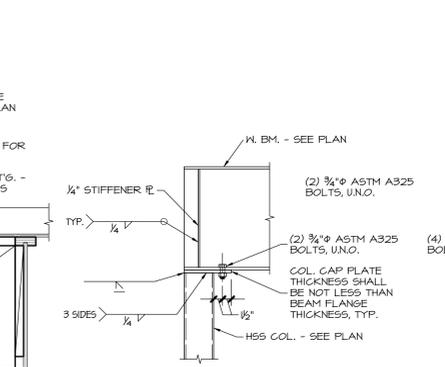
1 TYP. CHORD SPLICE
3/4" = 1'-0"



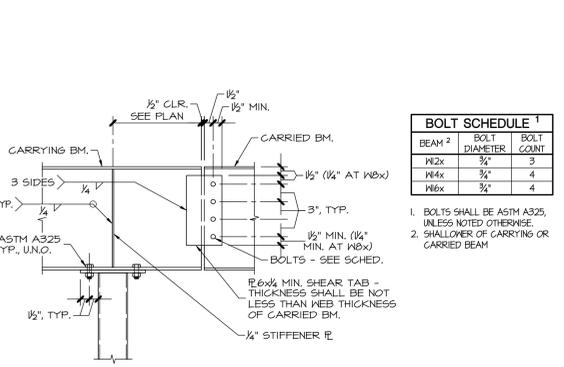
9 VENEER TO STUD WALL ATTACH
3/4" = 1'-0"



8 ROOF ACCESS HATCH
1" = 1'-0"



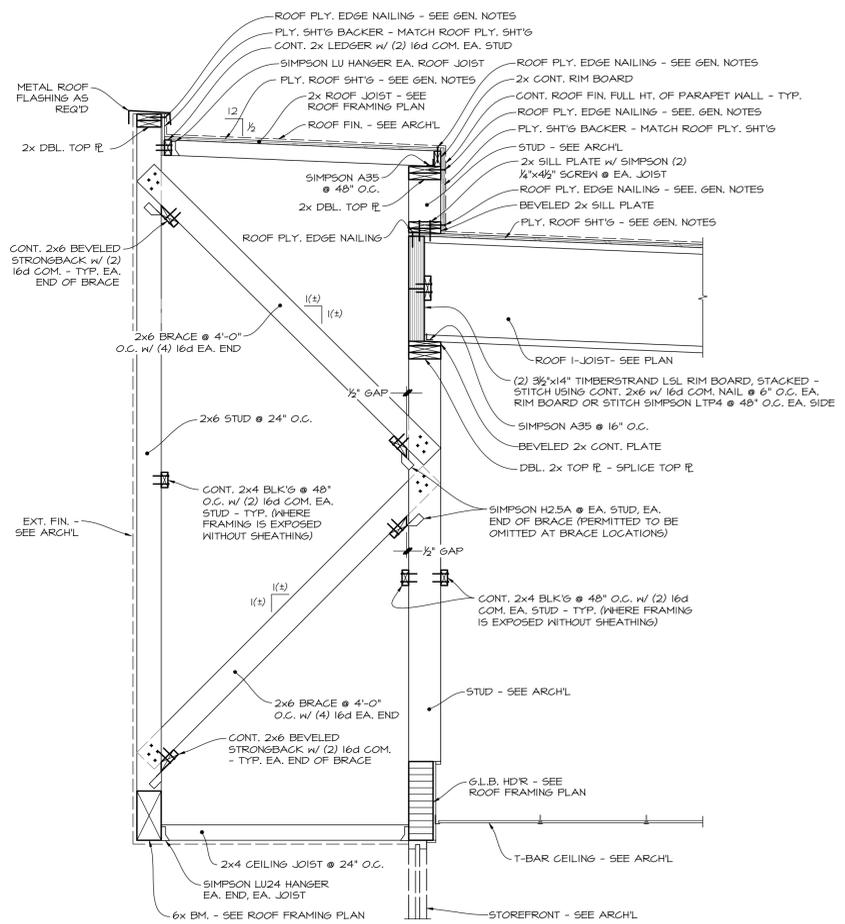
6 HSS COL. TO W.B.M. CONN.
1" = 1'-0"



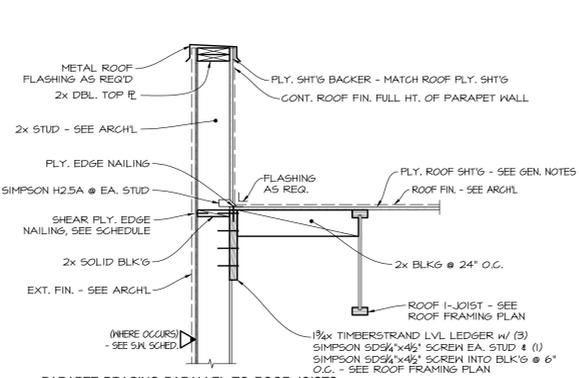
11 TYP. SHEAR WALL TRANSFER
1" = 1'-0"

BOLT SCHEDULE 1		
BEAM 2	BOLT DIAMETER	BOLT COUNT
M2x	3/4"	3
M4x	3/4"	4
M6x	3/4"	4

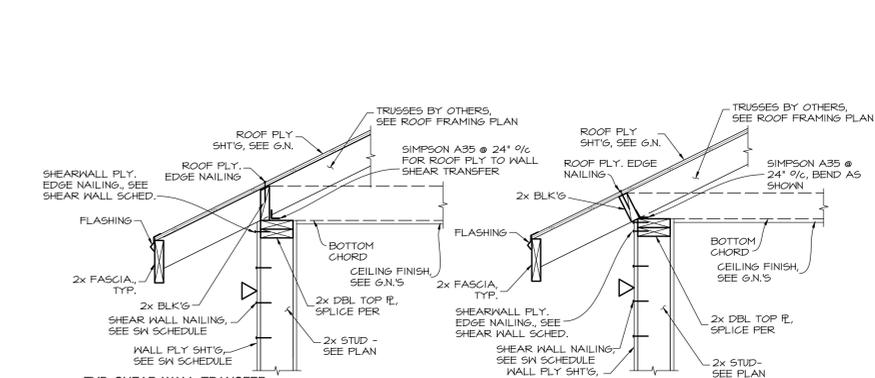
1. BOLTS SHALL BE ASTM A325, UNLESS NOTED OTHERWISE.
2. SHALL OVER OF CARRYING OR CARRIED BEAM.



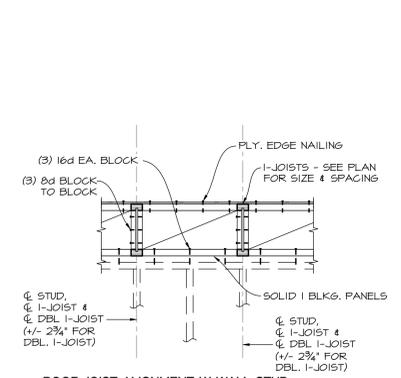
15 PARAPET FRAMING AT CUSTOMER ENTRY
3/4" = 1'-0"



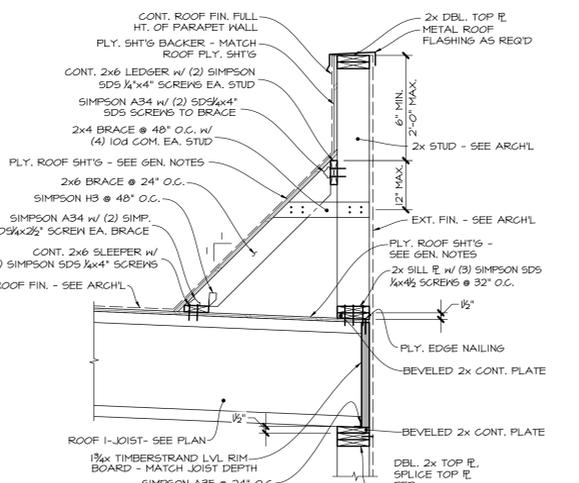
12 PARAPET BRACING PARALLEL TO ROOF JOISTS
3/4" = 1'-0"



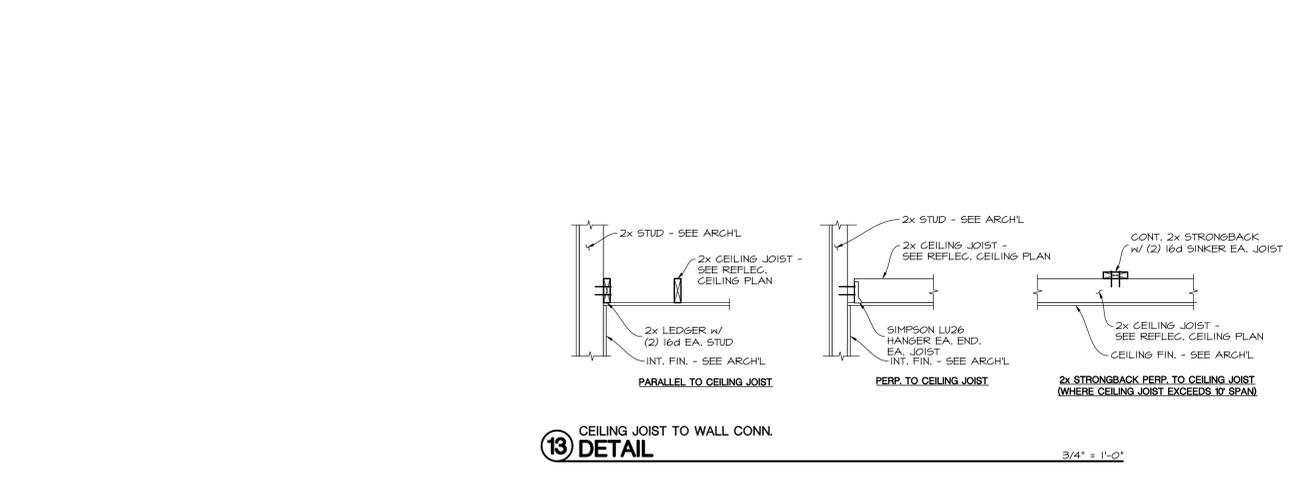
11 TYP. SHEAR WALL TRANSFER
1" = 1'-0"



10 ROOF JOIST ALIGNMENT W WALL STUD
3/4" = 1'-0"

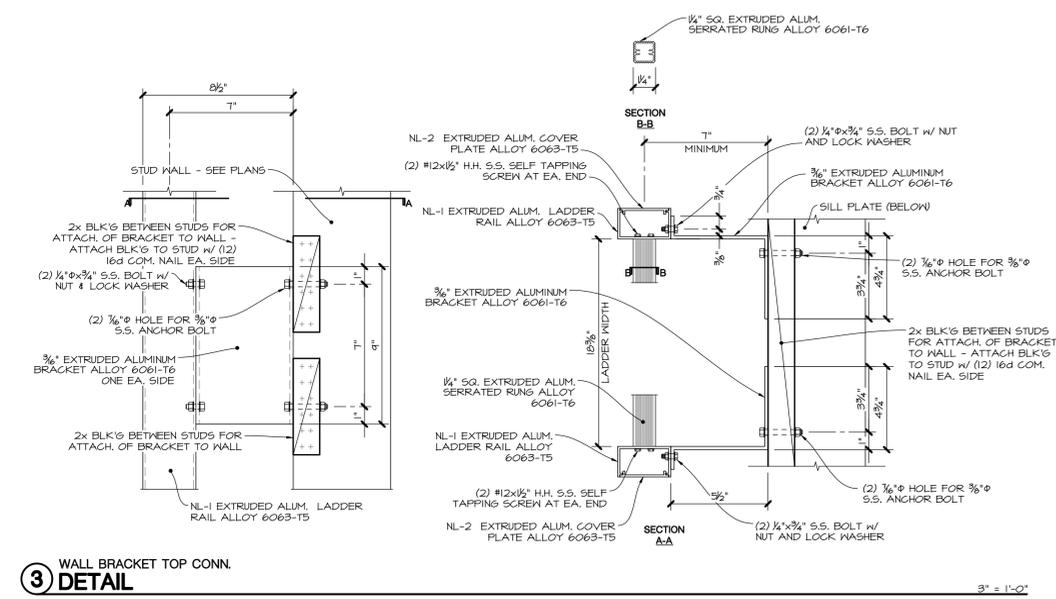
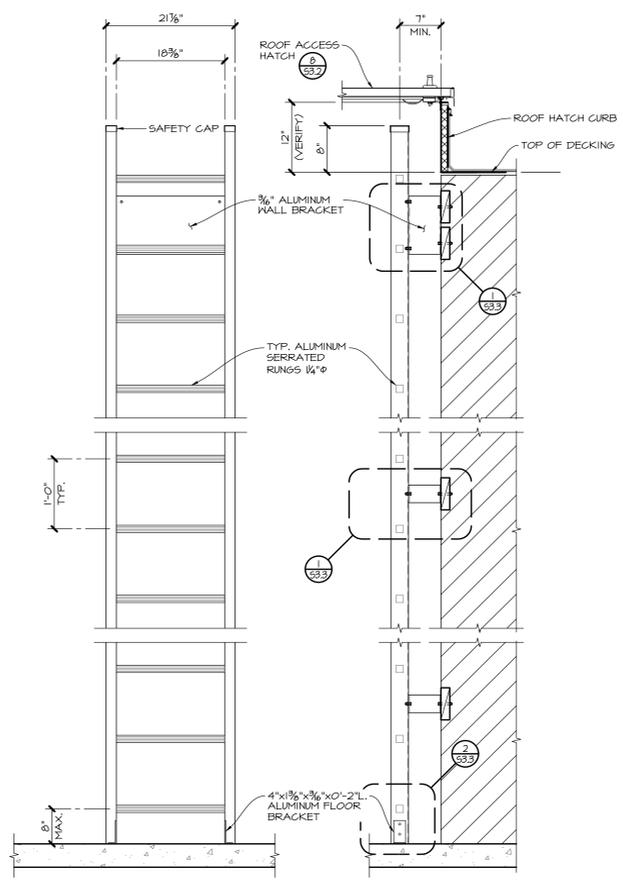
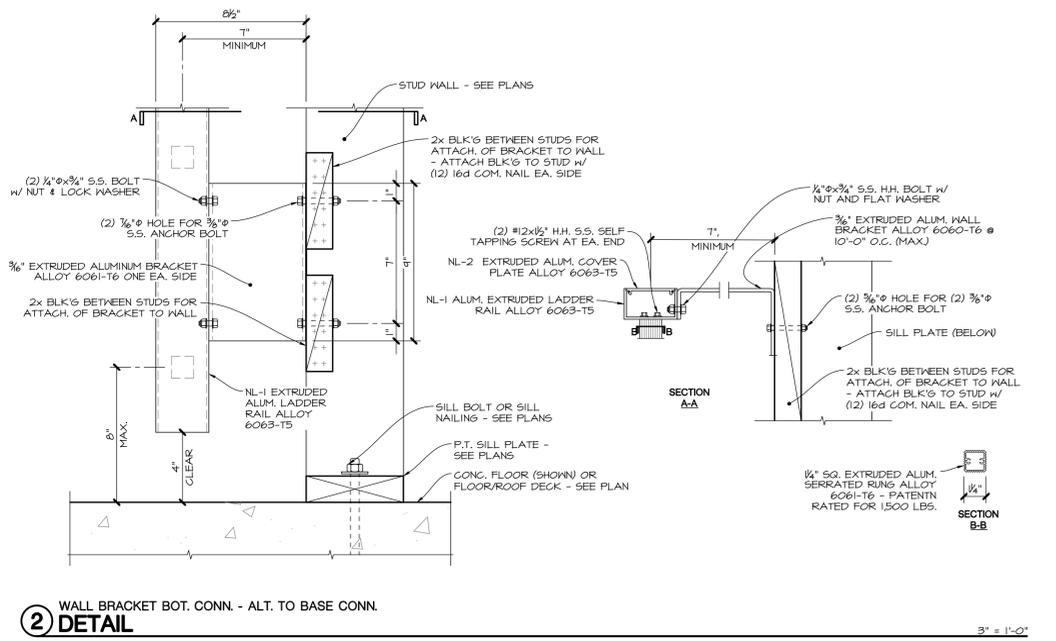
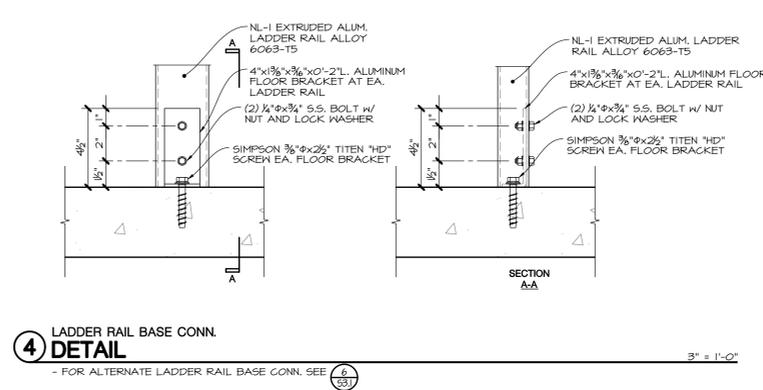
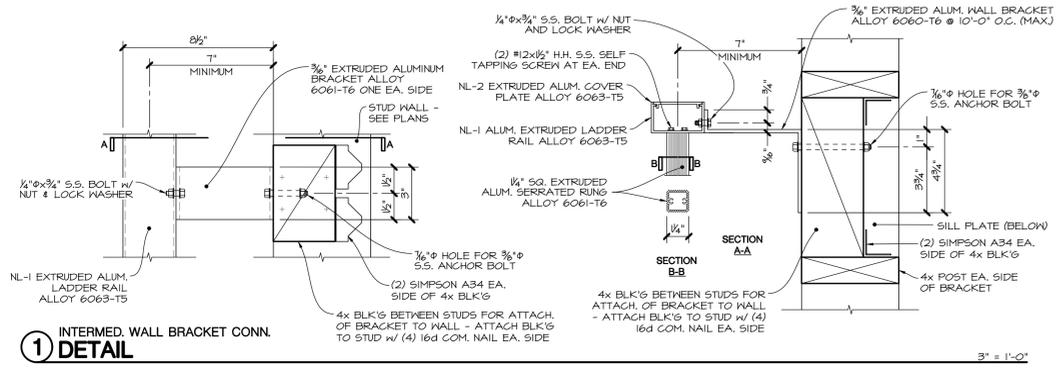


14 PARAPET BRACING PERP TO ROOF JOISTS AT LOW ROOF
3/4" = 1'-0"



13 CEILING JOIST TO WALL CONN.
3/4" = 1'-0"





ENGEL & COMPANY Engineers 4009 UNION AVENUE BAKERSFIELD, CA 93305 www.engeengineers.com (661) 327-7025		DRAWN: ECH DATE: 11/11/21 CHECKED: JH APPROVED: JH	Details Shopping Center Rancho Vista Shopping Center 3007 Rancho Vista Blvd. Palmdale, CA 93551	SHEET NO. S3.3 OF
DATE	ISSUED FOR			

File: P:\customers\architectural_group\32403 - rancho vista shopping center\32403\32403.dwg Plot Date: 11/11/21 10:52 AM
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